

## DYNAMIC COLLAPSE ANALYSIS OF FOUR-STORY STEEL FRAME USING E-SIMULATOR

Tomoshi Miyamura<sup>1\*</sup>, Makoto Ohsaki<sup>2</sup>, Takuzo Yamashita<sup>3</sup>,  
Daigoro Isobe<sup>4</sup>, Masayuki Kohiyama<sup>5</sup>

<sup>1</sup>Department of Computer Science, College of Engineering, Nihon University  
1 Nakagawara, Tokusada, Tamura, Koriyama 963-8642, Japan  
miyamura@cs.ce.nihon-u.ac.jp

<sup>2</sup>Department of Architecture, Hiroshima University  
1-4-1 Kagamiyama, Higashi-Hiroshima 739-8527, Japan  
ohsaki@hiroshima-u.ac.jp

<sup>3</sup>Hyogo Earthquake Eng. Res. Center, National Res. Inst. for Earth Science and Disaster Prevention  
1501-21 Nishikameyama, Mitsuta, Shijimi, Miki 673-0515, Japan  
tyamashi@bosai.go.jp

<sup>4</sup>Division of Engineering Mechanics and Energy, University of Tsukuba  
1-1-1 Tennodai, Tsukuba-shi, Ibaraki 305-8573, Japan  
isobe@kz.tsukuba.ac.jp

<sup>5</sup>Department of System Design Engineering, Keio University  
3-14-1 Hiyoshi, Kohoku, Yokohama 223-8522, Japan  
kohiyama@sd.keio.ac.jp

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**Abstract.** *Dynamic collapse analyses of a 4-story steel building frame modeled by solid elements are performed using the E-Simulator, which is a parallel finite element analysis software package for civil and building structures. Code verification, calculation verification, and validation of the numerical codes (V&V) are conducted by comparing the results obtained by the E-Simulator, those obtained by a beam element analysis code, and the experimental results obtained by the 3-D full-scale earthquake testing facility, the E-Defense at NIED. The seismic response analyses under 60% or 100% of the JR Takatori seismic wave of the 1995 Hyogoken-Nanbu earthquake are performed. The shear forces and interstory drift angles of the 1st story obtained by the E-Simulator simulation and the E-Defense full-scale test have good agreement. Both response of the whole frame and local deformation due to plastic buckling are simulated in the analysis using the E-Simulator.*

## 1 INTRODUCTION

The E-Simulator is a parallel finite element analysis software package for precise seismic response analysis of civil and building structures using solid elements [1]-[3]. The E-Simulator is under development in Hyogo Earthquake Engineering Research Center at National Research Institute for Earth Science and Disaster Prevention (NIED), Japan. A full-scale shake-table test for a 4-story frame (Fig. 1) was conducted using a 3-D full-scale earthquake testing facility, the E-Defense at NIED [5]. In this study, dynamic collapse analyses of the 4-story steel building frame modeled by solid elements are conducted using the E-Simulator. The seismic response analyses under 60% or 100% of the JR Takatori record of the 1995 Hyogoken-Nanbu earthquake are performed.

Validation of the E-Simulator is carried out by comparing the results obtained by the E-Simulator and the experimental results obtained by the E-Defense. The analyses using the ASI-Gauss code, which is a beam element analysis code, are also performed for the same frame and the both two codes are verified by comparing their results.

Tada et al. [6] conducted 3D analyses of the 4-story frame and compared the results with those by a full-scale shake-table test. Nam and Kasai [7] conducted 3D analyses of the 4-story frame using a fiber element method in which the effect of local buckling at the column ends is taken into account. The authors simulated the test of the 4-story steel frame using the E-Simulator [3]. Isobe et al. [8] performed analyses of the frame using an adaptive finite element code, the ASI-Gauss code, which is based on the beam element. They compared the results with those obtained by the E-Simulator [3] and the experimental results of the E-Defense test.

The analysis model used in the previous study [3] is improved in this study, that is, the stud bolts connecting the slabs and beams are considered to be rigid beams, the column bases are modeled precisely using solid elements and rigid beams, the Drucker–Prager yield criterion is introduced for concrete material used in the slabs, and a piecewise-linear combined isotropic-kinematic hardening model is introduced for the steel material.

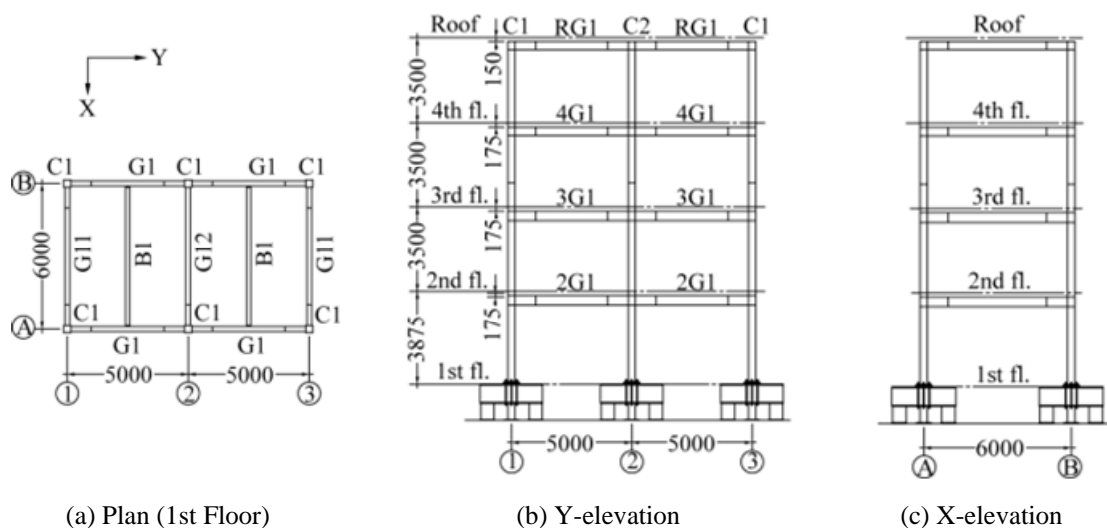


Figure 1: 4-story steel frame [4].

## 2 E-SIMULATOR

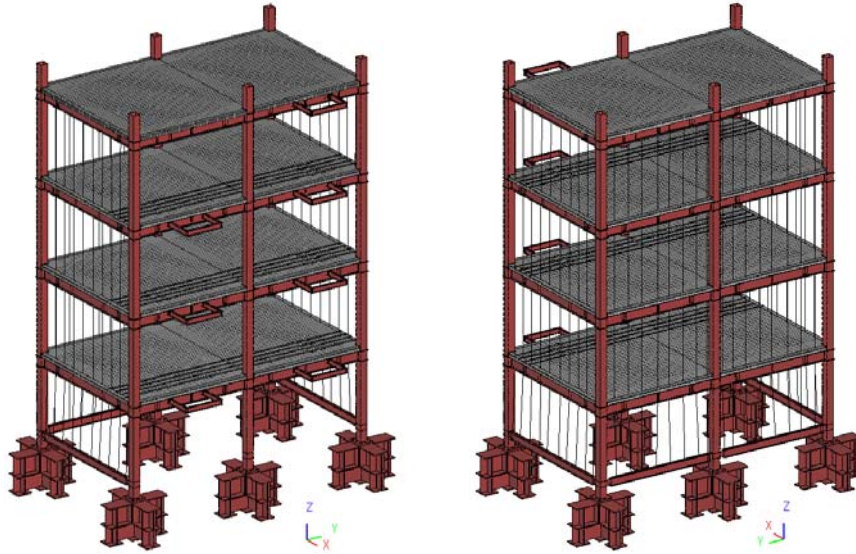
The E-Simulator is a parallel finite element structural analysis code for virtual shaking-table tests of civil or architectural structures. It enables large-scale analysis to be performed with a very fine mesh of solid elements. The main core of the E-Simulator is the commercial software package ADVENTURECluster [9, 10], which has been extended from the open source version, ADVENTURE [11]. These packages use the domain decomposition method for parallel implementation. The structural analysis code in the ADVENTURE system adopts the balancing domain decomposition (BDD) method, which is a substructuring-based linear iterative method with a Neumann-Neumann preconditioner combined with coarse grid correction. On the other hand, the Coarse Grid Conjugate Gradient (CGCG) method [10] has been developed originally for the ADVENTURECluster. The CGCG method is a conjugate gradient method combined with domain decomposition. It is preconditioned by motion of the decomposed subdomains. This idea is similar to the coarse grid correction used in the BDD method. However, the CGCG method is not a substructuring-based iterative method, and the computation cost for static condensation in each subdomain is reduced. The ADVENTURECluster can be operated in a massively parallel computer; indeed, it was implemented on Blue Gene/L in 2006 and the work was selected as a finalist in the 2006 Gordon Bell Prizes [10].

## 3 ANALYSIS MODEL FOR E-SIMULATOR

The FE model for the 4-story frame shown in Fig. 1 is generated using the data and documents distributed for the blind analysis contest [12]. In this study, the analysis model used in the previous study [3] is improved. All the members and the floor slabs are modeled by 8-node hexahedral solid elements; i.e., the DOFs of each node correspond to three translational displacements, and the displacements in the elements are interpolated by linear shape functions and incompatible modes. Figure 2 shows the FE mesh used in the present study.

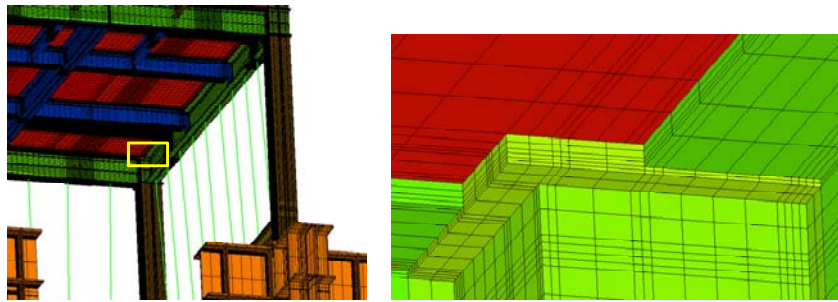
The FE mesh made by the Noguchi Laboratory at Keio University, Japan, is used as the prototype. The final mesh has 4,532,742 elements, 6,330,752 nodes, and 18,992,256 DOFs. Plates such as the flanges and webs of beams are divided into at least two layers of solid elements (Fig. 2(b)). Each floor slab is also divided into solid elements with two layers. Studs connecting the flange and the slab are modeled using rigid beam elements that are represented by a set of multi-point constraints as shown in Fig. 2(c). Steel bars in the slab are modeled by solid elements as shown in Fig. 2(c). The cross-section of these bars is represented approximately by rectangular shape. The column bases are modeled precisely using solid elements and rigid beam elements. The size of each element in the longitudinal direction of a beam or a column is approximately 13 mm near the connections, where severe plastic deformation is expected, while a coarser mesh is used for elements located far from the connections. The exterior walls are modeled by elastic-plastic shear springs. A piecewise-linear combined isotropic-kinematic hardening model [13] is introduced in the constitutive model for the steel material, and its parameters are determined from the uniaxial test results distributed for the blind analysis contest. The Drucker–Prager yield criterion is introduced for concrete material used in the slabs.

The self-weight of the steel is computed based on a mass density of  $7.86 \times 10^3 \text{ kg/m}^3$ . In contrast, the mass density of  $2.3 \times 10^3 \text{ kg/m}^3$  of the slab is increased appropriately to include the weights of nonstructural components, anti-collapse frames and stair landings installed in the experimental model. Five lowest natural periods for the analysis model are shown in Table 1. The Rayleigh damping with coefficients of  $\alpha = 0.251$  and  $\beta = 0.118 \times 10^{-3}$  is applied. These coefficients correspond to a damping factor of 0.02 for the 1st and 4th modes obtained by the eigenvalue analysis, which are the two lowest modes in the X-direction.



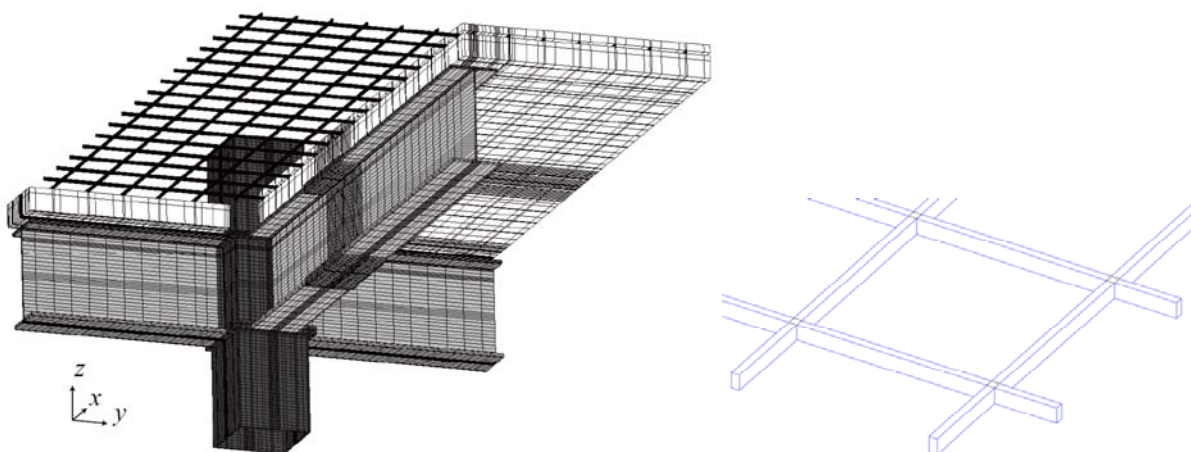
(a) Whole frame

(b)



(c) Close-up view

(d)



(e) Modeling of studs connecting the flange and the slab, and steel bars in the slab

Figure 2: Mesh of 4-story steel frame.

Table 1: Five lowest natural periods for analysis model and experimental model.

Model	Natural period				
	1st	2nd	3rd	4th	5th
Analysis model	0.8262	0.8081	0.5418	0.2660	0.2611
Experiment [5]	0.82	0.74–0.78	---	---	---

#### 4 ANALYSIS MODEL FOR ASI-GAUSS CODE

For verifying the E-Simulator code, the analysis using the beam elements based on the ASI-Gauss technique is also performed. The ASI-Gauss technique proposed by Lynn and Isobe [14] is an improved version of the adaptively shifted integration (ASI) technique developed by Toi and Isobe [15]. These two are adaptive finite element methods using beam elements and can be applied to the seismic response analysis of framed structures that exhibit collapse sequences to obtain highly accurate solutions with a small computational cost. In the ASI-Gauss technique, the accuracy of solutions is improved particularly in the elastic range compared to the ASI technique.

Figure 3 shows the beam element model for the ASI-Gauss code, which consists of 1,388 elements, 995 nodes and 5,970 DOFs. Each column and beam in the structure is modeled with two Timoshenko beam elements per member. Instead of considering the composite beam effect, the floor slabs in the structure are modeled as a braced truss with stiffnesses and weights approximated to actual values. Offsets of the positions of the slabs from the beams are considered by imposing appropriate vertical beams. The structural strengths of the stairs and the exterior walls are ignored, and only their masses are taken into account as a dead load. The bases are completely fixed to the ground, and the stiffness of the lowest columns are increased appropriately to match the experimental model. In the original calculation process of the ASI-Gauss technique, the bending moments are estimated at the exact locations of the nodes, that is, intersections of the center lines of the beams and columns. However, in the analysis performed in this study, the bending moments are estimated at the face of the connections to reduce the modeling error. Bilinear isotropic hardening is used in the constitutive model of the steel material, and its parameters are determined from uniaxial test results distributed in the blind analysis contest [12]. A mass proportional damping with a coefficient of  $\alpha = 0.2284$  is applied.

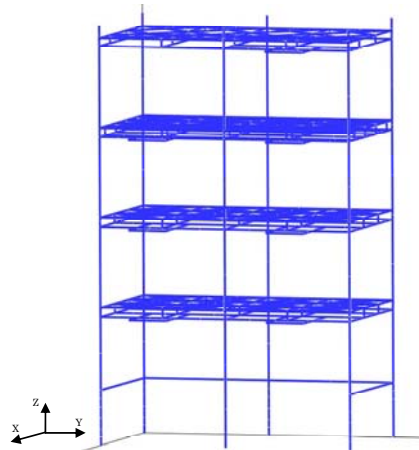


Figure 3: Beam element model of the four-story steel frame for the ASI-Gauss code.

## 5 RESULTS OF NUMERICAL SIMULATION

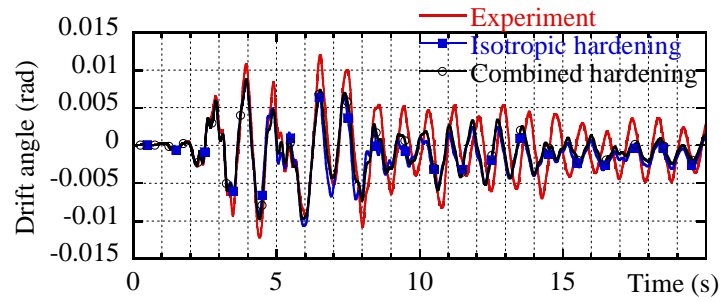
### 5.1 Seismic response analyses under 60% of the JR Takatori seismic wave

A 20-second segment of the JR Takatori wave is used as an input ground motion for the experiment performed in the E-Defense. The acceleration measured on the shaking table during the full-scale test under the 60 % of the JR Takatori wave is used for the input ground motion for both analyses using the E-Simulator and the ASI-Gauss code. The computational time using the E-Simulator took approximately one month using an SGI Altix 256-core computer, and the ASI-Gauss code took approximately 150 minutes using an Intel Core i7 2.93 GHz personal computer.

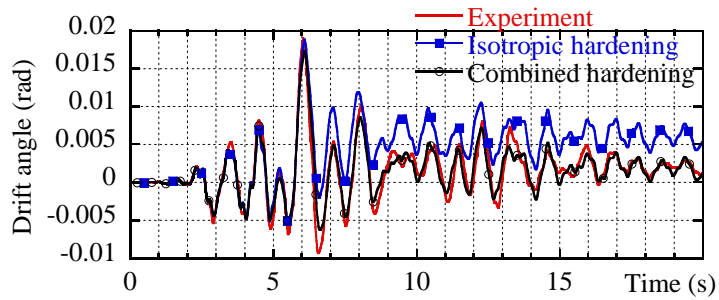
Figures 4 and 5 show the comparisons of the time histories of the interstory drift angle and the shear force of the 1st story, respectively, which are obtained by the E-Simulator and the E-Defense full-scale experiment [4]. For the E-Simulator, the results obtained by the combined hardening model for the steel material and those obtained by the isotropic hardening model are shown. The interstory drift angle is calculated from the lateral displacements at the center of the second floor. The shear force is calculated as the summation of the concentrated mass multiplied by the acceleration at the center of gravity of each floor. The interstory drift angle along the X-axis obtained by the E-Simulator, as shown in Fig. 4 (a), agrees well with the experiment until approximately 7 s. However, the differences in the phase and the amplitude become larger after 7 s, where the amplitude of the input ground motion decreases. The interstory drift angle along the Y-axis obtained by the E-Simulator, as shown in Fig. 4(b), shows a good resemblance to the experimental result when the combined hardening model is used. The interstory drift angle remains in the positive region in the later part of the time history when the isotropic hardening model is used. The time histories of the shear force shown in Fig. 5 seem to show similar tendencies as those of the interstory drift angle in Fig. 4.

Figures 6 and 7 show the comparisons of the time histories that are obtained by the E-Simulator, the ASI-Gauss code, and the E-Defense full-scale experiment. For the E-Simulator, only the results for the combined hardening model are shown. As described previously, the differences in the phase and the amplitude of the interstory drift angle along the X-axis obtained by the E-Simulator and the E-Defense become larger after approximately 7 s as shown in Fig. 6(a). The results obtained by the ASI-Gauss code, on the other hand, show good agreement with the experimental results in both the phase and the amplitude. However, the ASI-Gauss code has smaller peak values near 6 s. The time histories of the shear force shown in Fig. 7 seem to show similar tendencies as those of the interstory drift angle. It is noted that the asymmetrical drift in the positive direction in the later part of the time history of the interstory drift angle in the X-direction is not observed in the result obtained by the ASI-Gauss code though the isotropic hardening model is used in the analysis.

Figures 8 and 9 show the deformations and the distributions of the plastic region obtained by both analyses at 6.0 s. The deformation is magnified 10 times, and the colors represent the distribution of the equivalent stresses in Fig. 8 and the distribution of the yield function values in Fig. 9. The yield function values are linearly interpolated using the values at the both ends of the elements. Figure 8 shows that the plastic regions around connections are localized and the positions of the plastic regions are the same as the positions of the plastic hinges (indicated with red color) that are shown in Fig. 9. Therefore, the use of the plastic hinge model must be a good approximation of the plastic behavior obtained by the E-Simulator model. A torsional deformation can be clearly identified in the E-Simulator model, whereas less torsional deformation is observed in the ASI-Gauss model. The torsional deformation is not clearly observed in the full-scale experiment. Therefore, the stiffness of the shear spring used in the E-Simulator model for representing the exterior walls may be too large.

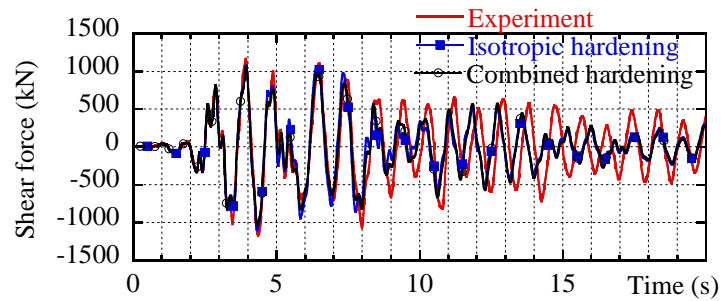


(a) X-direction

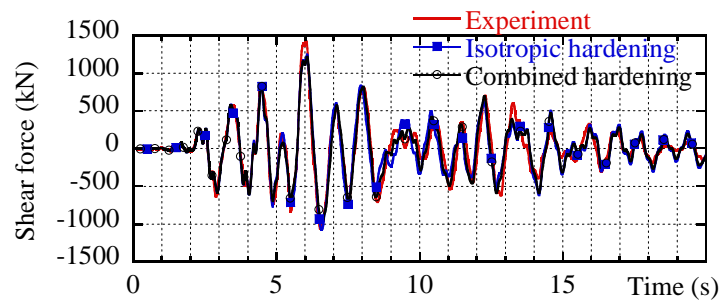


(b) Y-direction

Figure 4: Comparison of time-histories of interstory drift angle of the 1st story obtained by the E-Simulator and the E-Defense full-scale experiment.



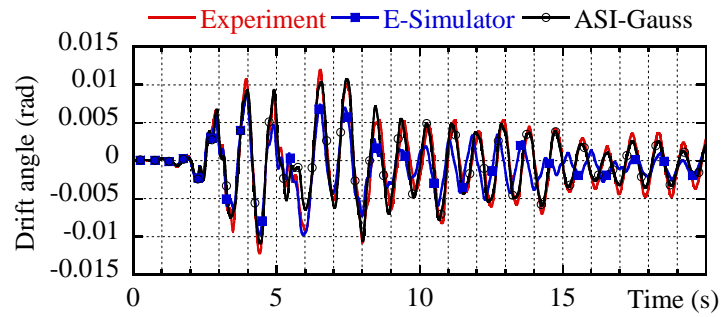
(a) X-direction



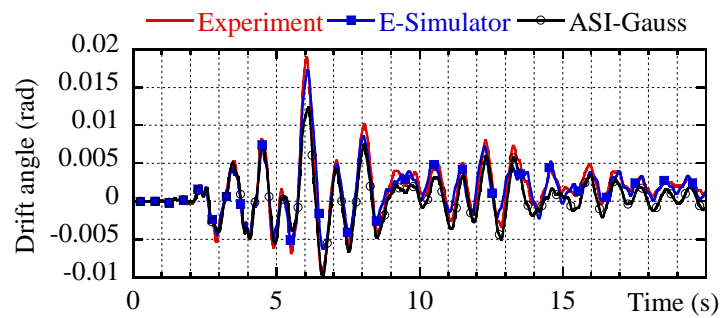
(b) Y-direction

Figure 5: Comparison of time-histories of shear force of the 1st story obtained by the E-Simulator and the E-Defense full-scale experiment.



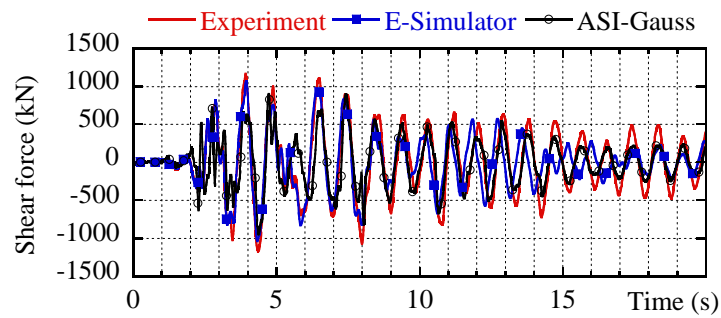


(a) X-direction

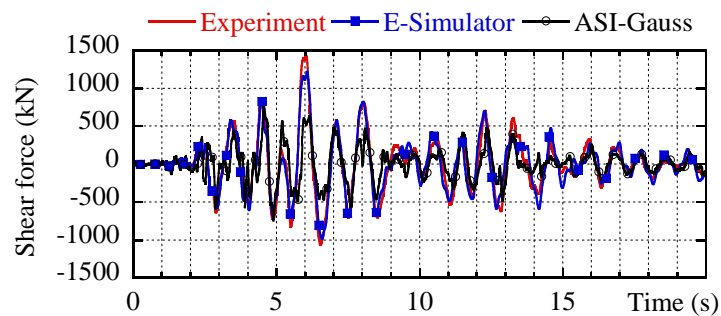


(b) Y-direction

Figure 6: Comparison of time-histories of interstory drift angle of the 1st story obtained by the E-Simulator and the ASI-Gauss code.



(a) X-direction



(b) Y-direction

Figure 7: Comparison of time-histories of shear force of the 1st story obtained by the E-Simulator and the ASI-Gauss code.



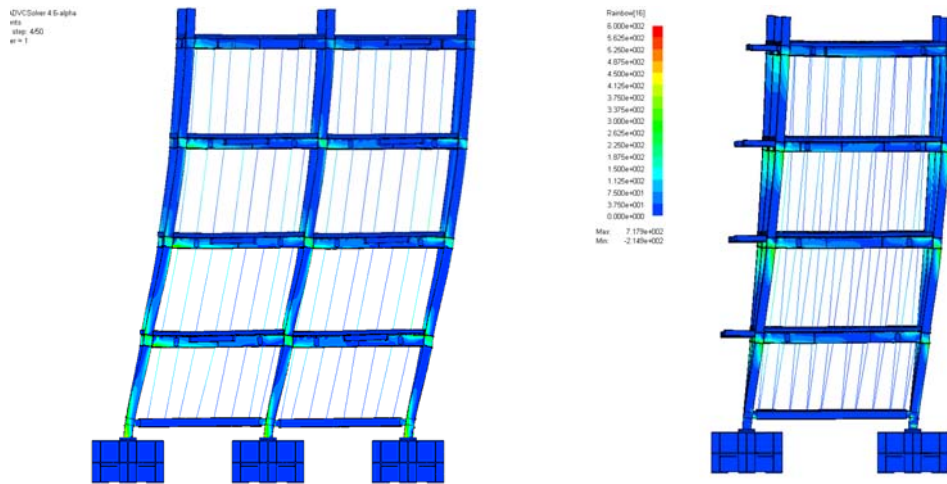


Figure 8: Deformation (magnified 10 times) and distribution of the equivalent stress at 6.08 s obtained by the E-Simulator.

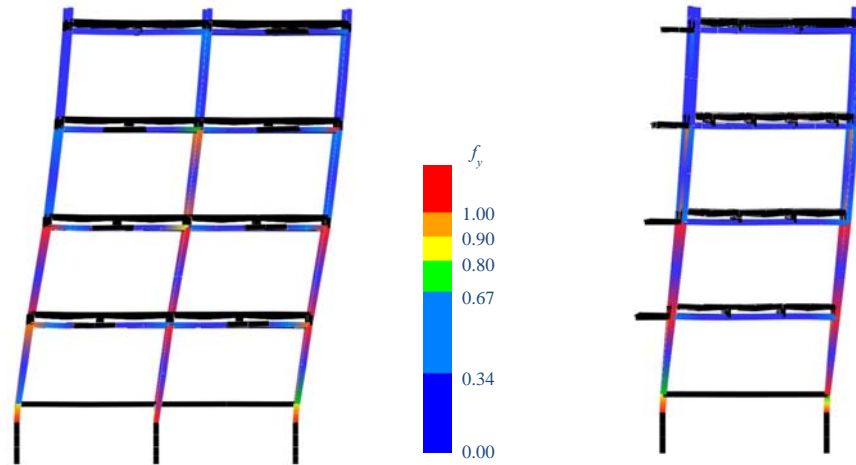
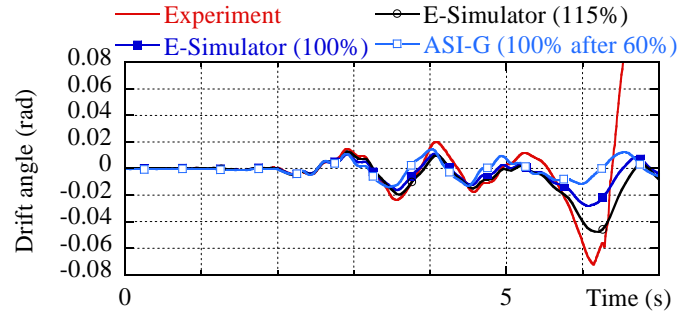


Figure 9: Deformation (magnified 10 times) and distribution of the yield function value at 6.0 s obtained by the ASI-Gauss code.

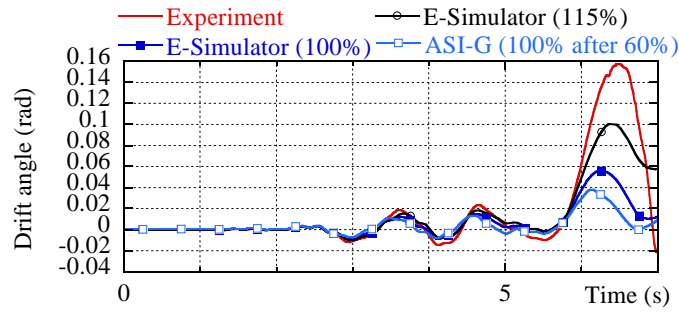
## 5.2 Seismic response analyses under 100% of the JR Takatori seismic wave

The acceleration measured on the shaking table during the full-scale test under the 100 % of the JR Takatori wave is used for the input ground motion in both analyses using the E-Simulator and the ASI-Gauss code. In the experiment, the shake-table test for 60% of the JR Takatori seismic wave is conducted before the test for the 100% of the wave is performed. Therefore, plastic deformation due to the preceding test remains in the specimen. In the simulation by the E-Simulation, however, the initial plastic deformation is not considered. In the simulation by the ASI-Gauss code, on the other hand, the preceding analysis for the 60% is performed. Figures 10 and 11 show the comparisons of the time histories of the interstory drift angle and the shear force of the 1st story, respectively, which are obtained by the E-Simulator, the ASI-Gauss code, and the E-Defense full-scale experiment. The simulation under 115% of the seismic wave is also conducted for the E-Simulator analysis model.

The frame collapsed after 6 s in the experiment and the interstory drift angle in the X-direction increased monotonically. Figure 12 shows deformation and distribution of the equivalent stress at 6.36 s obtained by the E-Simulator under 115% of the seismic wave. However, the frame is not fully collapsed in the analysis.

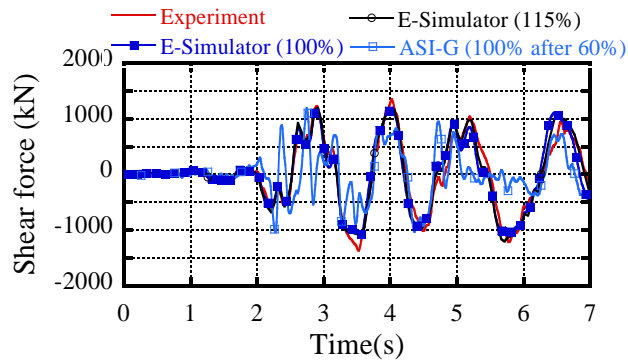


(a) X-direction

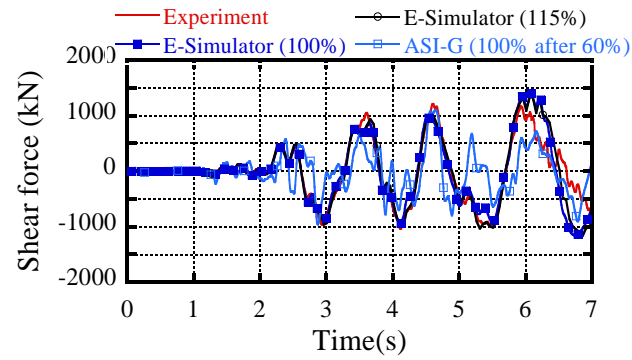


(b) Y-direction

Figure 10: Comparison of time-histories of interstory drift angle of the 1st story obtained by the E-Simulator and the ASI-Gauss code.



(a) X-direction



(b) Y-direction

Figure 11: Comparison of time-histories of shear force of the 1st story obtained by the E-Simulator and the E-the ASI-Gauss code.

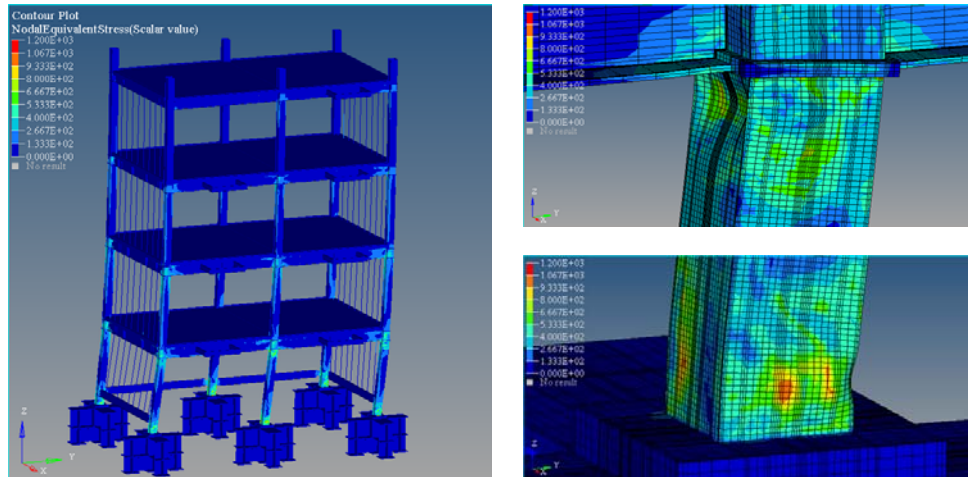


Figure 12: Deformation (not magnified) and distribution of the equivalent stress at 6.36 s obtained by the E-Simulator under 115% of the JR Takatori wave.

## 6 CONCLUDING REMARKS

Dynamic collapse analyses of the 4-story steel building, for which the full-scale shake table test was conducted at the E-Defense, are performed using the E-Simulator, which is parallel finite element analysis software for civil and building structures. The frame is modeled by solid elements. The analyses using the ASI-Gauss code, which is a kind of a beam element analysis code, are also conducted for the same steel frame.

In the analysis under the 60 % of the JR Takatori wave, the time histories of the interstory drift angle obtained by the E-Simulator show good agreement with the experimental results especially when the combined hardening model is used. However the differences in the phase and the amplitude of the time history of the interstory drift angle in the X-direction become larger after 7 s, where the amplitude of the input ground motion decreases. On the other hand, the results obtained by the ASI-Gauss code show good agreement with the experimental results in both the phase and the amplitude; however, it leads to smaller peak values near 6 s. In the analysis under the 100 % of the JR Takatori wave, both response of the whole frame and local deformation due to plastic buckling are simulated in the analysis using the E-Simulator.

## ACKNOWLEDGEMENTS

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## REFERENCES

- [1] M. Hori, H. Noguchi, T. Ine, Project report of development of numerical shaking table coping with E-defense. *JSCE Journal Earthquake Engineering*, **29**, 1420-1425, 2007 [in Japanese].
- [2] M. Ohsaki, T. Miyamura, M. Kohiyama, M. Hori, H. Noguchi, H. Akiba, K. Kajiwara, T. Ine, High-precision finite element analysis of elastoplastic dynamic responses of su-

- per-highrise steel frames. *Earthquake Engineering and Structural Dynamics*, **38**(5), 635-654, 2009.
- [3] T. Miyamura, M. Ohsaki, M. Kohiyama, D. Isobe, K. Onda, H. Akiba, M. Hori, K. Kajiwara, T. Ine, Large-scale FE analysis of steel building frames using E-Simulator. *Progress in Nuclear Science and Technology*, **2**, 651-656, 2011.
  - [4] Website of ASEBI at E-Defense, NIED, Project "E-Defense Tests on Full-Scale Four-Story Steel Building", <https://www.edgrid.jp/data/>, as of 28/2/2013.
  - [5] S. Yamada, K. Suita, M. Tada, K. Kasai, Y. Matsuoka, Y. Shimada, Collapse experiment on 4-story steel moment frame: Part 1 outline of test results. *Proceedings of the 14th World Conference on Earthquake Engineering* (DVD), Paper ID: S17.1.4, Beijing, China, 2008.
  - [6] M. Tada, H. Tamai, K. Ohgami, S. Kuwahara, A. Horimoto, Analytical simulation utilizing collaborative structural analysis system. *Proceedings of the 14th World Conference on Earthquake Engineering* (DVD), Paper ID: S17.1.5, Beijing, China, 2008.
  - [7] T.T. Nam, K. Kasai, Dynamic analysis of a full-scale four-story steel building experimented to collapse using strong ground motions, *Proceedings of International Symposium on Disaster Simulation and Structural Safety in the Next Generation 2011 (DS'11)*, 311-318, Kobe, Japan, 2011.
  - [8] D. Isobe, W.S. Han and T. Miyamura, Verification and validation of a seismic response analysis code for framed structures using the ASI-Gauss technique. *Earthquake Engineering and Structural Dynamics* (accepted).
  - [9] Website of Allied Engineering Corporation,  
<http://www.alde.co.jp/english/advc/index.html>, as of 28/2/2013.
  - [10] H. Akiba et al., Large scale drop impact analysis of mobile phone using ADVc on Blue Gene/L. *Proceedings of the International Conference on High Performance Computing Networking and Storage (SC06)*, Tampa, USA, 2006.
  - [11] S. Yoshimura, R. Shioya, H. Noguchi, T. Miyamura, Advanced general-purpose computational mechanics system for large scale analysis and design. *Journal of Computational and Applied Mathematics*, **149**(1), 279-296, 2002.
  - [12] M. Ohsaki, K. Kasai, Y. Matsuoka, J.Y. Zhang. Results of recent E-Defense tests on full-scale steel building: Part 2, Collapse simulation and blind analysis contest. *Proceedings of the Structures Congress 2008, ASCE*, Vancouver, Canada, 2008.
  - [13] M. Ohsaki, J.Y. Zhang, T. Miyamura, A heuristic algorithm for parameter identification of steel materials under asymmetric cyclic elastoplastic deformation. *Proceedings of the 7th China-Japan-Korea Joint Symposium on Optimization of Structural and Mechanical Systems (CJK-OSM7)* (CD-ROM), Huangshan, China, 2012.
  - [14] K.M. Lynn, D. Isobe, Finite element code for impact collapse problems of framed structures. *International Journal for Numerical Methods in Engineering*, **69**(12), 2538-2563, 2007.
  - [15] Y. Toi, D. Isobe, Adaptively shifted integration technique for finite element collapse analysis of framed structures. *International Journal for Numerical Methods in Engineering*, **36**(14), 2323-2339, 1993.